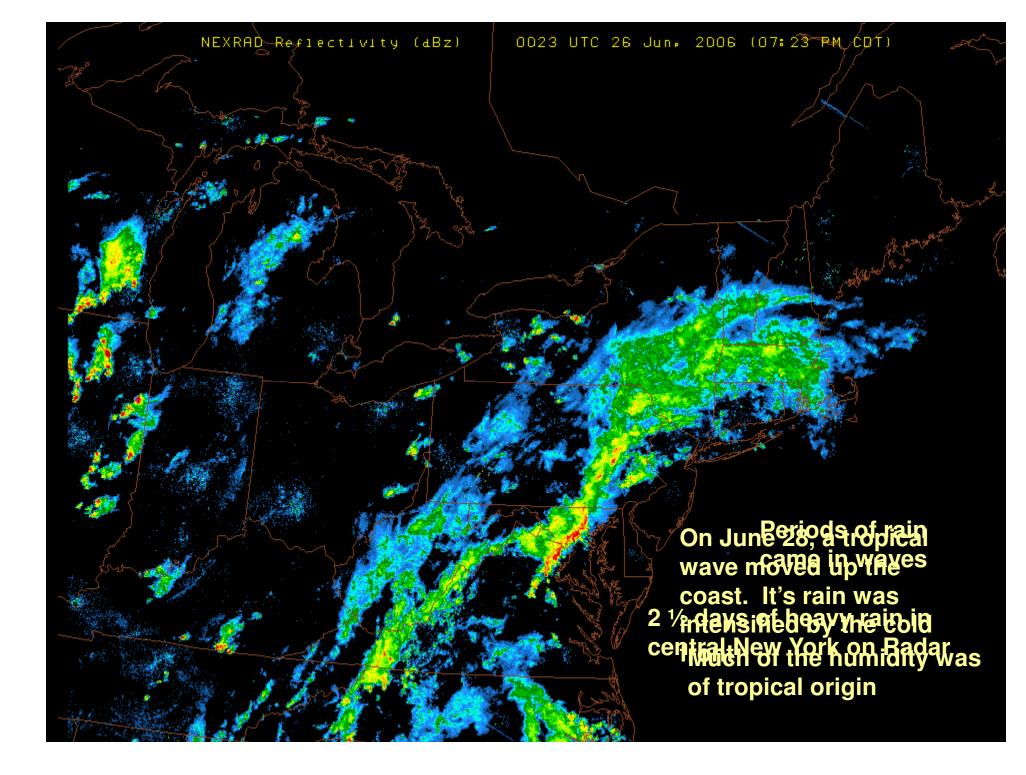
Recurrence Intervals for the June 2006 Flood in Delaware and Otsego counties, New York

Earth Sciences Brown Bag Seminar Spring 2008 Les Hasbargen Dept. of Earth Sciences SUNY Oneonta

NO STEP

Ouleout Creek, flood scars

Picture From : http://www.co.delaware.ny.us/flood 2006/Crop%20&%20Land%20Photos/default.htm





Floods caused erosion of cropland Significant transport of cobbles and boulders



Picture From : http://www.co.delaware.ny.us/flood 2006/Crop%20&%20Land%20Photos/default.htm



From: http://www.co.delaware.ny.us/flood 2006/During%20the%20Flood/default.htm



What can we learn from these floods?

- How often will a flood like this come along?
- What areas were most damaged?
- What areas were least affected?
- This was a "geomorphic" event. Can we learn something about erosion, transport, and deposition from this event?

There is a lot of basic research to do here!

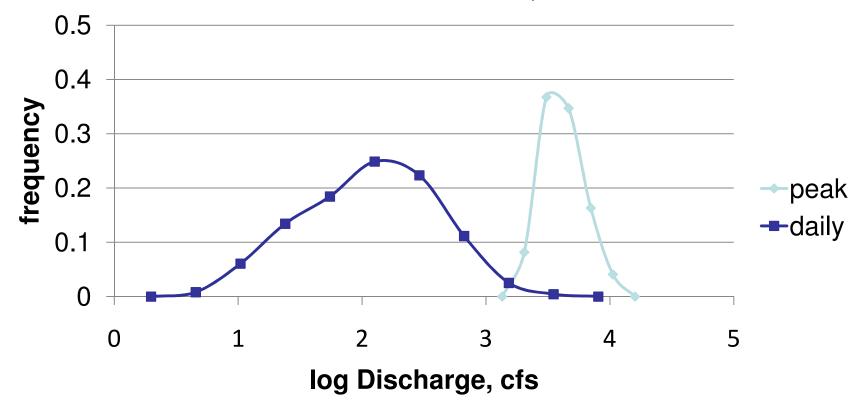
- Recent research by USGS provides estimates of *flood recurrence*—but there's more to be done...
- Quantify relationships between *rainfall and runoff* for local basins
- Characterize relation between *flood heights* and *channel characteristics* (how rough is the bed?)
- Document *erosion, transport, and deposition* in a range of settings and grain sizes
- Today, I'll start with flood recurrence

Flood Recurrence

- The frequency-size distribution for all of the flows a river experiences would be very helpful to know
- Once a mathematical model is fit to the data, we can determine the size of flow for a given recurrence interval (e.g., the 100yr flood)

Log-normal flow distributions

Flow distributions West Branch Delaware River, Delhi



A note on flood frequency distributions: Real flow distributions do not fit any known mathematical distributions perfectly well; the USGS recommends the Log Pearson Type III distribution, but they do so with a nod to future changes (USGS Circular 17b, 1982)

The standard model for flood frequency distributions from the USGS

- Treat the flood records as Log-normal distributions (Log Pearson Type III)
 - Compute average, standard deviation, and skew of log transformed data
- With these values, we can reconstruct the distribution with the LPTIII model

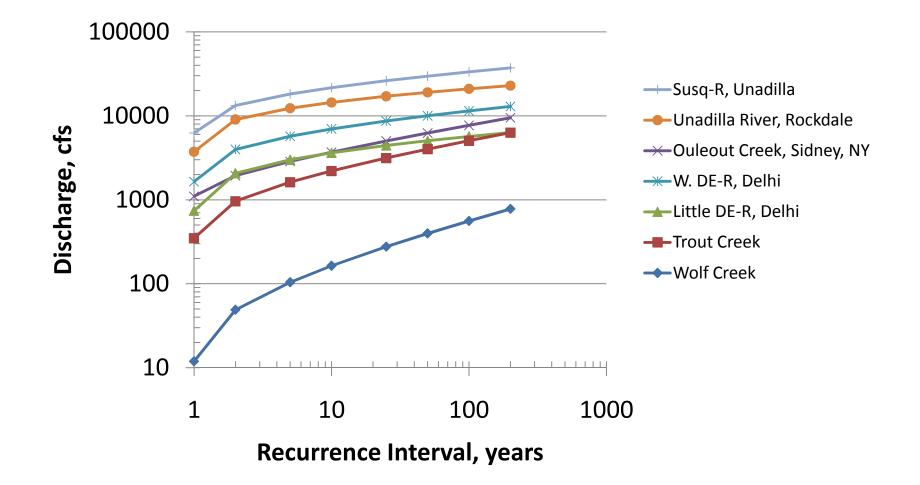
 $\log (Q) = \mu + K \sigma$

where μ is the average of log (*Q*), *K* is a frequency factor that depends on recurrence interval (and you need to use look-up tables to get this!!) and skew of log (*Q*) and σ is the standard deviation of log (*Q*)

• From this equation, we can estimate the probability that a flow of a given size will be equaled or exceeded

Recurrence Intervals Using Log Pearson Type III

Note Recurrence Interval is the reciprocal of probability



Flow frequency: Weibull method

- Flow events are ranked (m) from largest to smallest (the nth event)
- This is essentially the number of events in the record that are larger than some value [*exceedance probability*, *P*]
- P is the probability
- *m* is the rank
- *n* is the # data values
- *RI* is recurrence interval

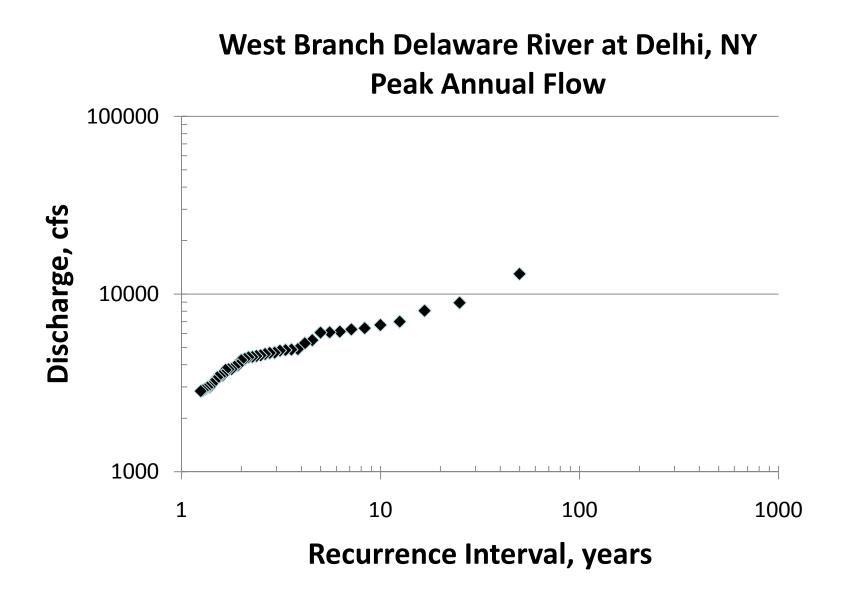
$$P = \frac{m}{n+1}$$
$$RI = \frac{1}{P} = \frac{n+1}{m}$$

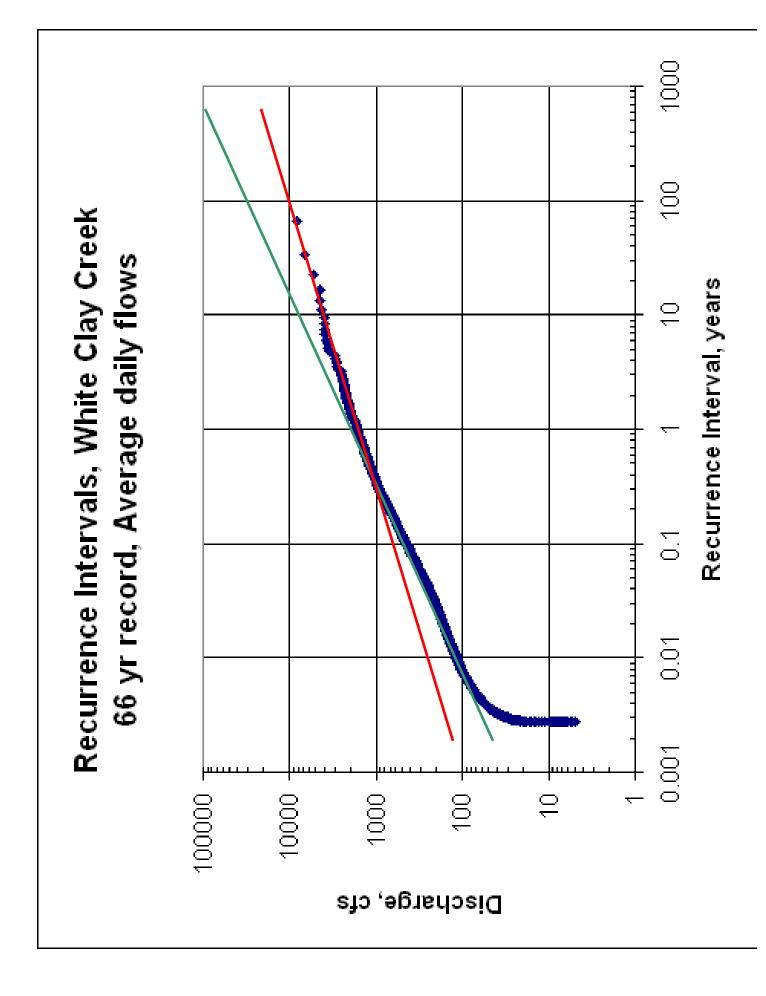
Basic steps to get the frequency

- Sort the data by size, largest to smallest
- Rank each event (*m*), from 1 (the largest) to *n* (the smallest)
- Calculate the probability *P* that a flow of a given size will be equaled or exceeded: $P = \frac{m}{n+1}$
- The reciprocal of *P* is the **recurrence interval** *RI*, the size of flow that comes along once in the time interval (years, in our situation) $_{RI} = \frac{n+1}{2}$

m

- Plot the discharge vs RI
- Fit a trend line to the data





Power law distribution (fractal)

- Many events in nature have lots of small events and few large ones (earthquakes, fault sizes, floods)
- These distributions are self-similar—we can use one part of the distribution to describe any other part
 - They look the same at any level of magnification
 - They can span several orders of magnitude [thresholds]
- A power law mathematical model describes such events
 - Order the flows and compute recurrence intervals
 - Fit a power law equation to the data
 - Bruce Malamud and Donald Turcotte, The applicability of power-law frequency statistics to floods, Journal of Hydrology Volume 322, Issues 1-4, 15 May 2006, Pages 168-180.
 - R. Kidson^{*}, K. S. Richards, & P. A. Carling[,] **Power-law extreme flood frequency**, *Geological Society, London, Special Publications*; 2006; v. 261; p. 141-153; DOI: 10.1144/GSL.SP.2006.261.01.11.

General form of a power law

 $y(x) = ax^b$

where *y* is the dependent variable

a is a constant coefficient; think of this as a unit scale (at x = 1) *x* is the variate

and *b* is the exponent which characterizes the rate of change of y vs x

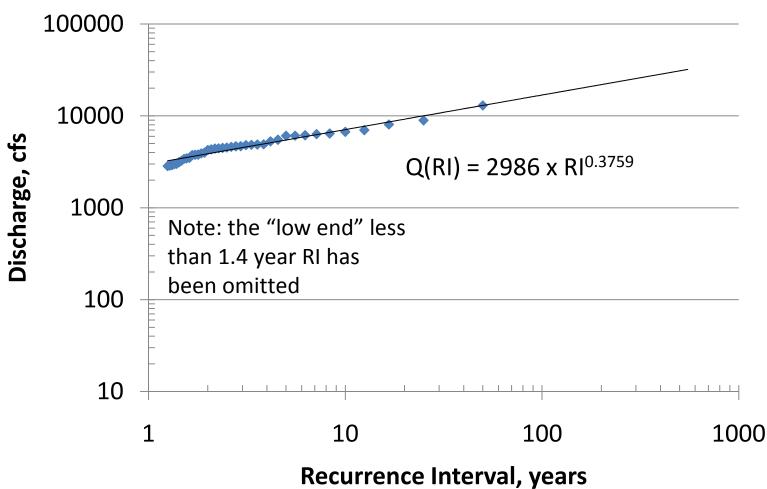
for instance, as $b \rightarrow 0$, y doesn't vary with x

if b > 0, then y increases with x

if b < 0, then y decreases with x

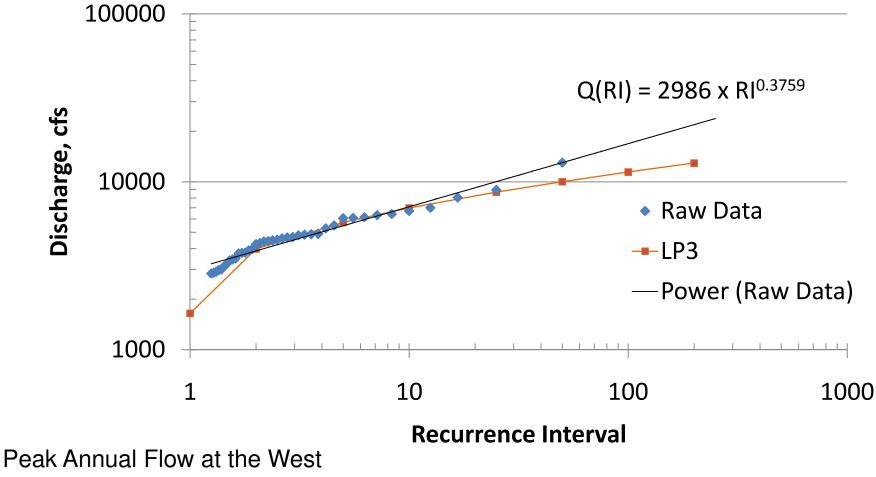
Power law flow distribution

West Branch Delaware River at Delhi, NY Peak Annual Flow



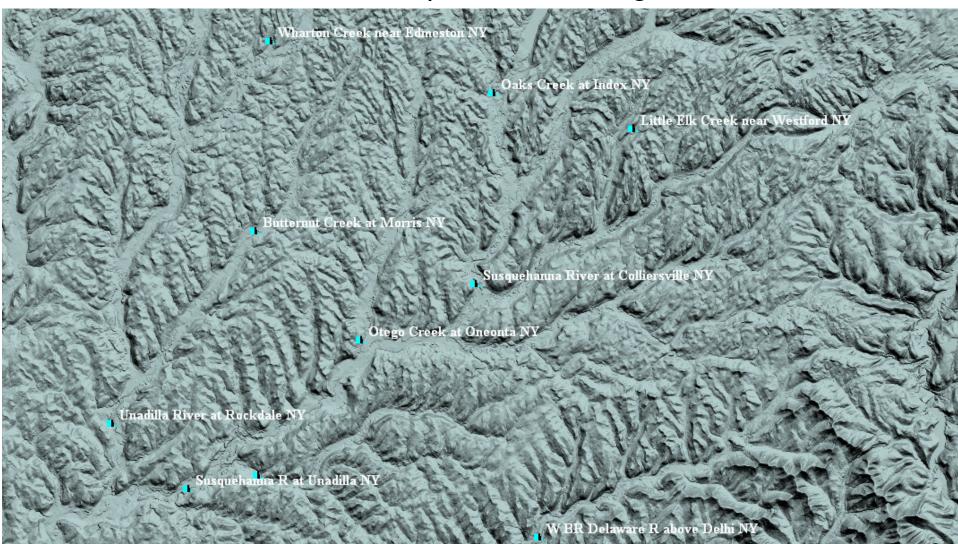
Note: Each gage record exhibits the same form as above, but with different exponents and coefficients in the power law relation

Comparing LPT3 and Power Law fits to the data



Peak Annual Flow at the Wes Branch Delaware River at Delhi, NY

Shaded Relief Map of USGS Gage Stations



Wolf Creek at Mundale NY

Trout Creek near Trout Creek NY

Recurrence Intervals for the 2006 Flood Event

River	Peak Discharge (cfs)	Recurrence Interval, LPT3	Recurrence Interval, Power Law
Susquehanna River, Unadilla	34,900	121 years	48 years
West Branch Delaware River, Walton	28,600	67 years	32 years
Unadilla River, Rockdale	23,100	167 years	70 years
West Branch Delaware River, Delhi	13000	208 years	51 years
Ouleout Creek	7250	76 years	43 years
Little Delaware River at Delhi	6100	141 years	50 years
Trout Creek	4350	54 years	25 years
Wolf Creek	350	23 years	12 years

Note: these are all model dependent approximations, and should NOT be considered an official estimate for planning purposes

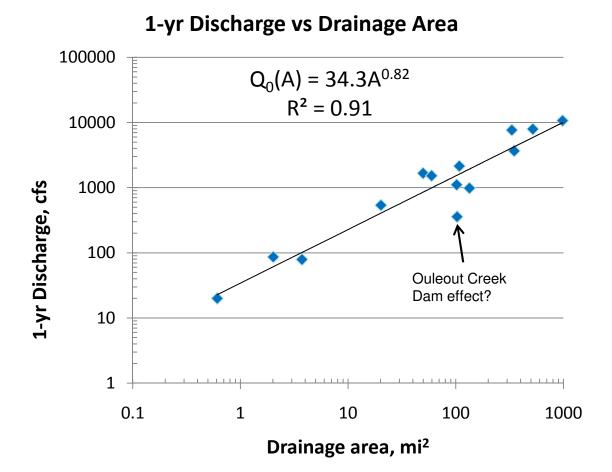
Bottom line on the 2006 Flood event

- The 2006 Flood is probably a once-(or twice)-in-a-lifetime experience, but this depends on where you live!!
- Recurrence intervals vary with drainage basin size (smaller basins exhibit larger ranges)
- The Log Pearson Type III distribution gives longer return periods than the power law
- The relationship of RI with basin size is worth following...

Flood Recurrence-Discharge power law parameters for Delaware and Otsego County

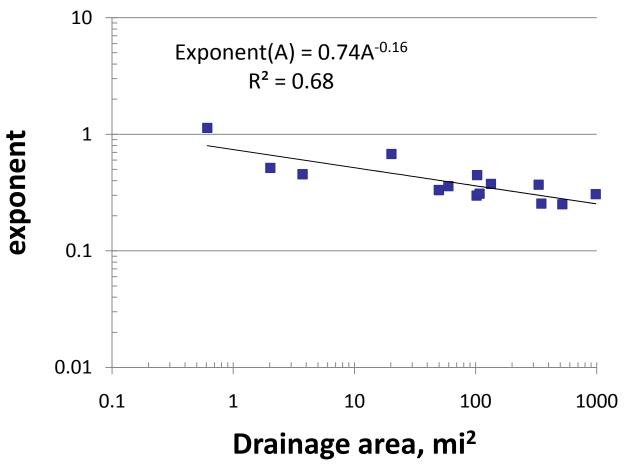
Drainage Basin	Area (mi²)	1-yr Discharge (cfs)	Scaling exponent
Wolf Creek	0.61	20	1.13
Wharton Creek, Edmeston NY	2.02	86.6	0.51
Little Elk Creek, Westford NY	3.73	79.2	0.45
Trout Creek	20.2	539	0.68
Little DE-R, Delhi	49.8	1669	0.33
Butternut Creek, Morris NY	59.7	1528	0.36
Oaks Creek, Index NY	102	1111	0.30
Ouleout Creek, Sidney, NY	103	359	0.45
Otego Creek, Oneonta NY	108	2149	0.31
W. DE-R, Delhi	134	986	0.38
W. Br. Del-R, Walton	332	7678	0.37
Susquehanna River, Colliersville	349	3677	0.25
Unadilla River, Rockdale	520	7956	0.25
Susq-R, Unadilla	982	10719	0.31

Plot of the power law coefficients vs drainage basin area for Delaware and Otsego Counties



The coefficient can be thought of as the 1-yr peak discharge for the given drainage basin

Plot of power law exponents vs drainage basin area for Delaware and Otsego Counties



The small value (-0.16) indicates a weak relation between drainage area and exponent; in general, smaller basins exhibit a larger exponent, which means that small basins exhibit larger increase in flood size with RI than large basins do

Predicting flow in ungauged basins

- We can use the empirical relations between power law parameters and basin size to develop a model for an ungauged basin
- First, compute the parameters given basin area
- Then these parameters go into the power law for discharge as a function of recurrence

Example: Ungaged basin model for Delaware and Otsego Counties

Given the two relations for power law parameters

 $Q_0(A) = 34.3A^{0.82}$

 $Exponent(A) = 0.74A^{-0.16}$

where A is an drainage basin area in mi²

If an ungaged basin is $A = 5 \text{ mi}^2$, then $Q_0(A) = 34.3 \times 5^{0.82} = 128$ $Exponent(A) = 0.74 \times 5^{-0.16} = 0.57$ So $Q(RI) = 128 RI^{0.57}$

Recent stream flow estimation from the USGS

• Multivariate analysis of stream flow, basin area, land cover and use, rainfall results in a set of predictive equations for ungaged basins

$$Q = kA^b S_{st}^{\ c} R^d S_{sl}^{\ e}$$

Lumia, Richard, Freehafer, D.A., and Smith, M.J., 2006, Magnitude and frequency of floods in New York: U.S. Geological Survey Scientific Investigations Report 2006–5112, 152 p.

- where k is a dimensional coefficient
- A is drainage area, in mi^2
- S_{st} is % storage in the drainage (lakes, wetlands, etc)
- *R* is mean annual rainfall, in inches
- S_{sl} is the ratio of stream slope to basin slope
- and b, c, d and e are scaling exponents derived empirically

USGS Recurrence Model for New York

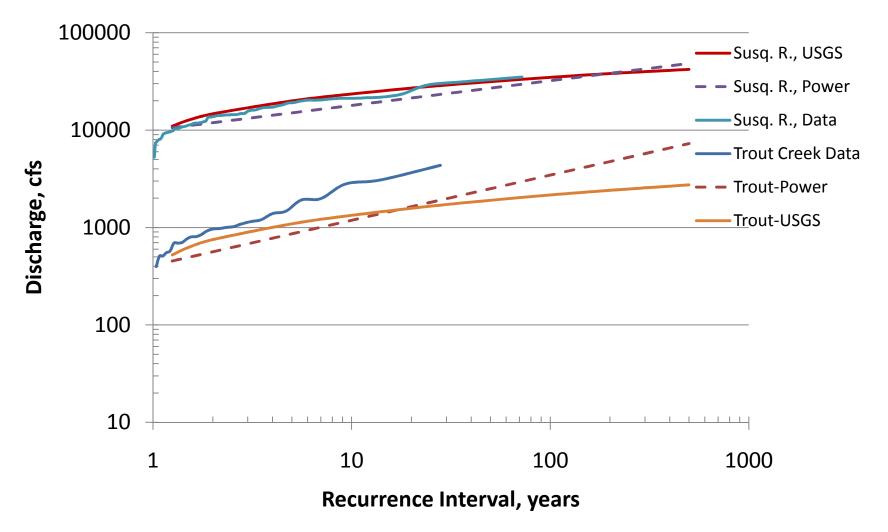
For ungauged basins, and based on Area (A), %storage (ST), annual rainfall (R), and channel to basin slope ratio (SR), and coefficients and exponents are functions of the recurrence interval for Log Pearson Type III distribution

Domion A

Kegion 4		
Q _{1.25}	=	0.037 (A) ^{1.029} (ST+0.5) ^{-0.104} (RUNF) ^{2.308} (SR) ^{0.317}
Q 1.5	=	0.064 (A) $^{1.022}$ (ST+0.5) $^{-0.120}$ (RUNF) $^{2.205}$ (SR) $^{0.320}$
Q ₂	=	0.115 (A) ^{1.012} (ST+0.5) ^{-0.139} (RUNF) ^{2.092} (SR) ^{0.319}
Q ₃	=	0.424 (A) ^{0.992} (ST+0.5) ^{-0.189} (RUNF) ^{1.822} (SR) ^{0.316}
Q 10	=	0.829 (A) ^{0.981} (ST+0.5) ^{-0.219} (RUNF) ^{1.685} (SR) ^{0.314}
Q 25	=	1.585 (A) ^{0.970} (ST+0.5) ^{-0.250} (RUNF) ^{1.559} (SR) ^{0.312}
Q 50	=	2.330 (A) ^{0.963} (ST+0.5) ^{-0.269} (RUNF) ¹⁴⁸⁹ (SR) ^{0.312}
Q 100	=	3.243 (A) ^{0.957} (ST+0.5) ^{-0.285} (RUNF) ¹⁴³¹ (SR) ^{0.312}
Q 200	=	4.350 (A) 0.952 (ST+0.5) -0.300 (RUNF) 1.380 (SR) 0.313
Q 500	=	6.163 (A) ^{0.946} (ST+0.5) ^{-0.317} (RUNF) ^{1.320} (SR) ^{0.315}

$Q(RI) = f(RI) \times A^{f(RI)} (S_T + 0.5)^{f(RI)} R^{f(RI)} S_R^{f(RI)}$

Unguaged Model Comparison USGS and Power Law



Conclusions

- Recurrence intervals for the 2006 flood event vary from 23 to 208 years
- Small basins exhibit shorter recurrence intervals
- Peak annual flow and average daily flow are different distributions (choice of distribution matters!)
- Models can be tweaked to fit local gage records quite well
- A power law model for flow distributions is simpler than the current USGS method
- We can estimate flows for ungaged basins, but crudely...

More work to do...

- Discrepancies between models need to be explored
- Establish relations for rainfall-runoff for local basins
- Survey 2006 flood heights, channel geometry, and channel changes in targeted settings
- Estimate flows in targeted field sites
- Get a better handle on flow depth and channel characteristics



Bedrock vs Alluvial cover in Morris Brook, about 100 m between locations

Flood heights varied dramatically between sites

Thanks for your Attention!



References

- Link to OSU's tutorial site for computing river flow statistics <u>http://water.oregonstate.edu/streamflow/</u>
- Link to USGS's site for the method to determine flood recurrence (LPT3)

http://water.usgs.gov/osw/bulletin17b/bulletin 17B.html

 Lumia, Richard, Freehafer, D.A., and Smith, M.J., 2006, Magnitude and frequency of floods in New York: U.S. Geological Survey Scientific Investigations Report 2006–5112, 152 p.